### **City of Molalla** Clackamas County, Oregon

# WASTEWATER TREATMENT PLANT 2017 LAGOON TEST REPORT

JULY 2017





# The Dyer Partnership Engineers & Planners, Inc.

1330 Teakwood Ave. Coos Bay, Oregon 97420 (541) 269-0732 www.dyerpart.com 759 W. Central Ave. Sutherlin, Oregon 97479 (541) 459-4619 Project No. 100.29

1165 S. Park St. Lebanon, Oregon 97355 (541) 405-4520

# **City of Molalla** Clackamas County, Oregon

# Wastewater Treatment Plant 2017 Lagoon Leakage Test Report

## July 2017

Project No. 100.29





The Dyer Partnership Engineers & Planners, Inc.

1165 S. Park St. Lebanon, OR 97355 (541) 405-4520 www.dyerpart.com

### **Table of Contents**

- 1. INTRODUCTION
- 2. LAGOON PROCESS
- 3. TEST PERIOD
- 4. DATA COLLECTION METHODS AND EQUIPMENT
- 5. **RESULTING DATA**
- 6. SEEPAGE CALCULATIONS
- 7. CONCLUSION

### APPENDICES

### Appendix A

Figure A - Wastewater Treatment Plant Flow Schematic

### Appendix B

Lagoon Design Properties and Survey

### Appendix C

Data Collection Table

### Appendix D

Data Entry and Analysis Spreadsheets

### Appendix E

Oregon DEQ Guidelines for Estimating Leakage from Existing Sewer Lagoons

### 1. INTRODUCTION

Leak testing for the City of Molalla's wastewater lagoons was performed by The Dyer Partnership, Engineers and Planners, Inc. in July of 2017. The leak testing was conducted in accordance with the Oregon DEQ *Guidelines for Estimating Leakage for Existing Sewer Lagoons*.

### 2. <u>LAGOON PROCESS</u>

The Molalla Wastewater Treatment Plant (WWTP) process utilizes two lagoons, in series, for primary wastewater treatment. The data collection for the lagoon leak test was completed during normal plant operation, for both lagoons simultaneously. Based on the fact these lagoons work in series, it was not possible to isolate the individual lagoons during the testing period. Bypassing Lagoon No. 1 would create a short circuiting of flow in Lagoon No. 2 and result in inadequate treatment. The WWTP flow through the lagoons is noted below and is shown on the schematic included as Appendix A of this report:

- 1. Flows from the plant Headworks gravity flow to an Aeration Basin.
- 2. Flows from the Aeration Basin are pumped to Lagoon No. 1. Pumped flows to Lagoon No. 1 are metered by an 18-inch magnetic flow meter.
- 3. Flows from Lagoon No. 1 gravity flow to Lagoon No. 2 through a 14-inch diameter line.
- 4. Flows from Lagoon No. 2 gravity flow to two Dissolved Air Flotation (DAF) Units. Each DAF unit is served by a 14-inch gravity line, which are flow metered by magnetic flow meters on each line.

The physical properties for the lagoons from the 2008 NPDES Permit Renewal Application documentation (Tetra Tech) are found in Table 1, below.

Tuble 1. Lugoon Tioperues								
	Max.Surface AreaDepthat 6-foot depth		Lagoon Perimeter	Lagoon Side	Volume (6-foot depth to 12-foot depth)			
	(ft.)	(ac.)	(s.f.)	(ft.)	Slope	(gal.)	(gal./ft.)	(gal./in.)
Lagoon No. 1	12	11.4	496,584	3,380	3:1	23,651,940	3,941,990	328,499
Lagoon No. 2	12	13.6	592,416	3,480	3:1	27,993,272	4,665,545	388,795
		]	Totals			51,645,212	8,607,535	717,294

Table 1: Lagoon Properties

On July 10, 2017 Project Delivery Group completed a topographic survey of the Molalla lagoons. The purpose of the survey was to establish the water elevation and associated water surface areas of both lagoons at the start of the 10-day test period. The surface of Lagoon No. 1 was found to have an elevation of 320.04 feet (NAVD 88) and a total surface area of 521,014. sq. ft. The surface of Lagoon No. 2 had a surface elevation of 312.20 feet (NAVD 88) with a total surface area of 629,893 sq. ft. The storage volume at the test period depth of Lagoons No. 1 and

No. 2 is 324,765 gal./in and 392,634 gal./in, respectively. Therefore, the lagoons are calculated to have a total storage volume of **717,399 gal./in** at a total surface area of both lagoons of 1,150,907 sq. ft. This closely matches the volume per depth value of 717,294 gal./in noted in Table 1 and will be used for the calculations in this report.

### 3. <u>TEST PERIOD</u>

Data for the leakage analysis was collected over a ten day period starting Monday, July 10, 2017 and ending Thursday, July 20, 2017. Data collection occurred at 8:00 am each morning, starting with the initial readings on July 10<sup>th</sup>. Therefore, "Day 1" represents the 24 hour period between 8:00 am, July 10<sup>th</sup> and 8:00 am, July 11<sup>th</sup>. The subsequent days follow the same 24 hour period, from 8:00 am to 8:00 am the following day.

### 4. DATA COLLECTION METHODS AND EQUIPMENT

The equipment and methods used to collect data for the analysis of the leak testing included:

- Influent Lagoon Flow An 18-inch magnetic flow meter was installed on the effluent line of the transfer pump station that delivers wastewater from the Aeration Basin to Lagoon No. 1. The meter was installed in September of 2016 to assist in the data collection for this test. It should be noted that this meter was installed directly downstream of a vertical 45 degree bend. The City was able to verify from the manufacturer that the installation would produce less than one percent (1%) error in flow readings.
- Precipitation One rain gauge with 1/100 of an inch accuracy is located at the Wastewater Treatment Plant was available to record rain fall.
- Lagoon No. 1 Level The initial lagoon level was established using the staff gauge in Lagoon No. 1 with an associated distance measurement from the top of the Lagoon No. 1 effluent structure to the water surface level. All measurements were taken to the nearest 1/16 of an inch from the same location on the effluent structure, down to the water surface level.
- Lagoon No. 2 Level The initial lagoon level was established using the existing, sloped staff gauge in Lagoon No. 1. The staff gauge is angled up 16 degrees from the horizontal. Each morning a tack was installed at the water surface elevation on the staff gauge and the distance from this tack to the previous day tack was measured to the nearest 1/16 of an inch. The change in lagoon depth was calculated using this measurement and the 16 degree slope of the gauge.
- Lagoon Effluent Flow Two existing 14-inch magnetic flow meters were used to record the effluent flow from Lagoon No. 2 to the Dissolved Air Flotation (DAF) Units.
- Evaporation A 4-foot diameter evaporation pan with a Novalynx Analog Output Evaporation Gauge was used to record evaporation. This pan is recommended in the Oregon DEQ *Guidelines for Estimating Leakage for Existing Sewer Lagoons*.

### 5. <u>RESULTING DATA</u>

### A. Influent Flow

The total wastewater influent flow to the lagoons was 10,272,062 gallons over the 10-day test period, as recorded by the magnetic flow meter on the effluent transfer pump station line. Based on the volume of 717,399 gal./in. in the lagoons at or near the surface test period surface elevations, this equates to an increase in lagoon level of 14.32 inches over both lagoons, or **1.432 inches per day**. The daily flow data is listed in Table 2, below.

Table 2. Lago	on Influent Flow	
	Daily Flov	V
Day	(gal.)	(in.)
Day 1 (10/11 – 10/12)	234,465	0.33
Day 2 (10/12 – 10/13)	888,239	1.24
Day 3 (10/13 – 10/14)	898,910	1.25
Day 4 (10/14 – 10/15)	852,863	1.19
Day 5 (10/15 - 10/16)	890,909	1.24
Day 6 (10/16 - 10/17)	816,052	1.14
Day 7 (10/17- 10/18)	916,188	1.28
Day 8 (10/18 - 10/19)	866,121	1.21
Day 9 (10/19 - 10/20)*	1,809,927	2.52
Day 10 (10/20 - 10/21)*	2,098,388	2.92
Total	10,272,062	14.32

Table 2: Lagoon Influent Flow

\* Lagoon influent flow increased on days 9 and 10 due to the recirculation of wastewater from the DAF units to the aeration basin and lagoons.

### **B.** Precipitation

There was no measurable precipitation during the 10-day test period.

### C. Lagoon Levels

Over the testing period Lagoon No. 1 had an overall surface level elevation increase of 5.3 inches, which equates to an increase in storage volume of 1,721,257 gallons. Lagoon No. 2 had an overall surface level elevation decrease of 3.18 inches, which equates to a decrease in storage volume of 1,248,575 gallons. Therefore, there was a net storage amount of 472,682 gallons in the lagoons, which produces an overall increase in lagoon depth of 0.659 inches over both lagoons during the test period or **0.066 inches per day**. Daily lagoon depths are shown in Tables 3 and 4.

	Tuble 5. Eugobil 10. T Levels							
	Change	in Depth	Lagoon Depth					
	(in.)	(ft.)	(ft.)					
7/10/17	-	-	9.40					
7/11/17	0.00	0.000	9.40					
7/12/17	-1.20	-0.100	9.30					
7/13/17	0.75	0.063	9.36					
7/14/17	0.06	0.005	9.37					
7/15/17	-0.37	-0.031	9.34					
7/16/17	-0.63	-0.053	9.28					
7/17/17	-0.06	-0.005	9.28					
7/18/17	0.50	0.042	9.32					
7/19/17	2.50	0.208	9.53					
7/20/17	3.75	0.313	9.84					
Totals	5.30	0.442	-					

Table 3: Lagoon No. 1 Levels

### Table 4: Lagoon No. 2 Levels

	Change	in Depth	Lagoon Depth
	(in.)	(ft.)	(ft.)
7/10/17	-	-	10.10
7/11/17	-0.62	-0.12	9.98
7/12/17	-0.69	-0.08	9.90
7/13/17	-1.17	-0.09	9.81
7/14/17	-0.31	-0.13	9.68
7/15/17	-0.41	-0.03	9.65
7/16/17	1.52	0.13	9.78
7/17/17	1.45	0.12	9.90
7/18/17	-1.43	-0.12	9.78
7/19/17	-0.28	-0.02	9.76
7/20/17	-1.24	-0.10	9.66
Totals	-3.18	265	-

### **D.** Effluent Flow

The total effluent flow from the lagoons to the DAF units during the 10-day test period was 6,673,000 gallons, as recorded by the two DAF unit magnetic flow meters. Based on the volume of 717,399 gal./in. in the lagoons (Table 1), this equates to a decrease in lagoon levels of 9.30 inches, or **0.930 inches per day**, over both lagoons during the test period. The daily effluent flow data is listed in Table 5, below.

17	able 5: Lagoon Ellivent Flow						
		Dail	y Flow				
	DAF No	. 1 Meter	DAF No	. 2 Meter			
Day	(gal.)	(in.)	(gal.)	(gal.)			
Day 1 (10/11 – 10/12)	95,000	0.13	197,000	0.27			
Day 2 (10/12 – 10/13)	209,000	0.29	496,000	0.69			
Day 3 (10/13 – 10/14)	263,000	0.37	687,000	0.96			
Day 4 (10/14 – 10/15)	259,000	0.36	592,000	0.83			
Day 5 (10/15 - 10/16)	267,000	0.37	705,000	0.98			
Day 6 (10/16 - 10/17)	201,000	0.28	454,000	0.63			
Day 7 (10/17- 10/18)	0	0.00	0	0.00			
Day 8 (10/18 - 10/19)	81,000	0.11	306,000	0.43			
Day 9 (10/19 - 10/20)	222,000	0.31	566,000	0.79			
Day 10 (10/20 - 10/21)	264,000	0.37	809,000	1.13			
Totals	1,861,000	2.59	4,812,000	6.71			
Combined Total (gal.)		6,67	/3,000				
Combined Total (in.)		(	9.3				

### E. Evaporation

The measured evaporation during the 10-day test period was 2.452 inches. A correction factor of 0.8 was applied to the pan evaporation. The standard correction factors for pan coefficients are typically 0.7 to 0.9. The resulting evaporation within the lagoons was 1.97 inches or **0.197 inches per day** over the test period.

#### 6. <u>SEEPAGE CALCULATIONS</u>

#### A. Flow Balance

The calculations for determining the seepage from the lagoons was derived through a simple flow balance for the system using the inches per day flows determined above. The equation used in the calculations is:

Seepage (in./day) = Influent (in./day) + Precipitation (in./day) – Lagoon Storage (in./day) – Effluent (in./day) – Evaporation (in./day)

The equation above results in:

**Seepage** (in./day) = 1.432 in./day + 0 in./day - .066 in./day - 0.930 in./day - 0.197 in./day = **0.239 inches per day** 

#### **B.** Volume Check

As a check to the above flow balance calculation, a 10-day volume balance is shown below in Table 7. Flows contributing to influent flow to the lagoons are shown as positive numbers and flows contributing to effluent and storage within the lagoons are shown as negative. The balance of these volumes is considered as the total amount of seepage during the test period, and is found to be 1,719,130 gallons over the 10-day test period or 171,913 gallons per day. Based on the storage the lagoons of 717,399 gallons per inch, the balance value is equal to seepage amount of 2.40 inches or **0.240 inches/day**.

	olulle Dululle	6
Flow Element	Total Flow 10 Day Test Period (gal.)	Avg. Daily Flow 10 Day Test Period (gal./day)
Lagoon Influent Flow (Gal.)	10,272,062	1,027,206
Precipitation (Gal.)	0	0
Lagoon 1 Storage (Gal.)	-1,721,257	-172,126
Lagoon 2 Storage (Gal.)	1,248,575	124,857
Lagoon Effluent (DAF No. 1) (Gal.)	-1,861,000	-186,100
Lagoon Effluent (DAF No. 2) (Gal.)	-4,812,000	-481,200
Evaporation (Gal.)	-1,407,250	-140,725
Balance/Seepage (Gal.)	1,719,130	171,913

Table 7: Lagoon Volume Balance

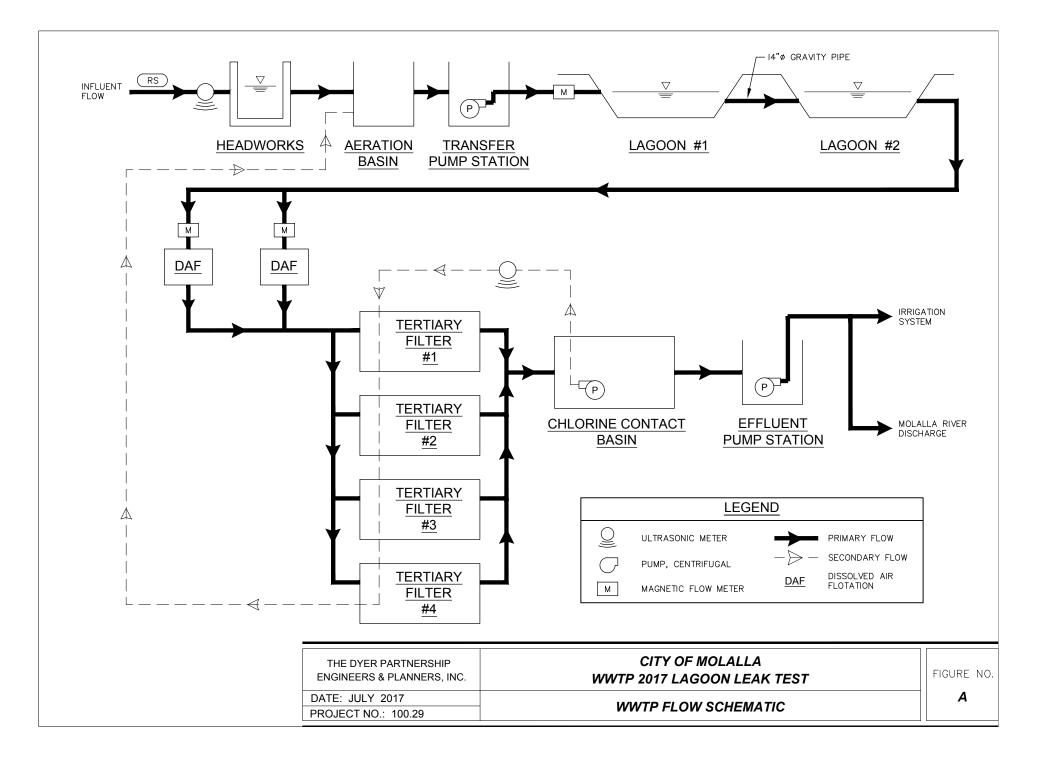
### 7. <u>CONCLUSION</u>

The guidelines for estimating leakage from existing sewage lagoons produced by the Oregon DEQ states that seepage rates as high as 1/8 of an inch per day or less are considered normal. Seepage exceeding <sup>1</sup>/<sub>4</sub>-inch per day indicates a seal failure, or absence of adequate initial seal.

The lagoon leakage test did not exceed <sup>1</sup>/<sub>4</sub>-inch per day. The lagoon leakage test **PASSED.** 

### Appendix A

Figure A – Wastewater Treatment Plant Flow Schematic



### Appendix B

Lagoon Design Properties and Survey

ADWF - Average dry weather flow   0.80 min.     MMDWF - Average wet weather flow   1.28 min.     AWWF - Average wet weather flow   1.30 min.     MWWWF - Max month wet weather flow   2.04 min.     MWWF - Peck day flow   7.06 min.     DESIGN DATA   BOD5     Effluent Quality   BOD5     Anticipated Filter Effluent Quality   BOD5     Headworks (2002 Construction)   Type of screens   1     Number of screens   1     Peak flow capacity, each   9.25 min.     Bypass screen   Manua     Screenings washing   Yes     Screenings compaction   Yes     Screenings washing   Yes     Screation   10 <t< th=""><th>ngd 1.1 m ngd 1.7 m ngd 2.3 m ngd 3.1 m ngd 8.5 m &lt; 10 mg/l &lt; 5 mg/l smnel fine screet ngd ly cleaned scre ed hes ngd ngd set by 54 feet t with 2 feet fi 000 gallons :concrete ing rsepower w weir to pump con)</th><th>ngd 4.1 mgd ngd 10.3 mg TSS &lt; 10 mg/l TSS &lt; 5 mg/l ns, perforated plate</th></t<>	ngd 1.1 m ngd 1.7 m ngd 2.3 m ngd 3.1 m ngd 8.5 m < 10 mg/l < 5 mg/l smnel fine screet ngd ly cleaned scre ed hes ngd ngd set by 54 feet t with 2 feet fi 000 gallons :concrete ing rsepower w weir to pump con)	ngd 4.1 mgd ngd 10.3 mg TSS < 10 mg/l TSS < 5 mg/l ns, perforated plate
Existing and Projected Flows   200     ADWF - Average dry weather flow   0.80 minimizes     AWWF - Average wet weather flow   1.38 minimizes     AWWF - Average wet weather flow   1.30 minimizes     AWWF - Average wet weather flow   1.30 minimizes     AWWF - Average wet weather flow   1.30 minimizes     AWWF - Max month wet weather flow   2.04 minimizes     PDF - Peak day flow   7.06 minimizes     DESIGN DATA   Effluent Quality   BOD5     Headworks (2002 Construction)   Type of screens   1     Type of screens   1   1     Peak flow capacity, each   9.25 minimizes   9.25 minimizes     Bypass screen   Manual   Screenings compaction   Yes     Screenings compaction   Yes   Screenings compaction   Yes     Screenings compaction   Yes   Screation   1     Minimum flow capability   0.27 minimum   1     Headworks, influent flow measurement   Number of flumes   1     Number of flumes   1   1   1     Throat width   24 inc   1   1     Dinensions   Size (bottom of basin)	ngd 1.1 m ngd 1.7 m ngd 2.3 m ngd 3.1 m ngd 8.5 m < 10 mg/l < 5 mg/l smnel fine screet ngd ly cleaned scre ed hes ngd ngd set by 54 feet t with 2 feet fi 000 gallons :concrete ing rsepower w weir to pump con)	ngd 1.4 mgd ngd 2.3 mgd ngd 3.0 mgd ngd 4.1 mgd 10.3 mg TSS < 10 mg/l TSS < 5 mg/l ns, perforated plate
MMWWF - Max month wet weather flow   2.04 million     PDF - Peak day flow   7.06 million     DESIGN DATA   Effluent Quality   B0D5     Anticipated Effluent Quality   B0D5     Anticipated Filter Effluent Quality   B0D5     Headworks (2002 Construction)   In-cha     Type of screens   In-cha     Number of screens   1     Peak flow capacity, each   9.25 million     Bypass screen   Manua     Screenings compaction   Yes     Screenings compaction   Yes     Screenings compaction   Yes     Number of flumes   1     Throat width   24 inc     Peak flow capacity   21.4 inc     Number of flumes   1     Throat width   24 inc     Peak flow capacity   0.27 in     Minimum flow capability   0.27 in     Maximum side water depth   10 feat     Basin volume, maximum   1,300,     Basin outlet   Construction)     Dimensions   200 ff     Side slopes (horizivert)   211     Maximum side water depth   10 feat  <	ngd 3.1 m ngd 8.5 m < 10 mg/l < 5 mg/l innel fine screen ngd lly cleaned scre ed hes ngd ngd ngd set by 54 feet t with 2 feet fi 000 gallons :concrete ing rsepower w weir to pump con)	ngd 4.1 mgd ngd 10.3 mg TSS < 10 mg/l TSS < 5 mg/l ns, perforated plate
Effluent Quality Required Effluent Quality Anticipated Filter Effluent Quality Headworks (2002 Construction) Type of screens Number of screens Peak flow capacity, each Bypass screen Screenings washing Screenings compaction Septage Headworks, influent flow measurement Number of flumes Throat width Peak flow capacity Minimum flow capability Aeration Basin (1980 Construction) Dimensions Size (bottom of basin) Side slopes (horiz:vert) Basin volume, maximum Basin liner Aerators Type Maximum side water depth Horsepower, each Basin outlet Transfer Pump Station (2002 Construction) Estimated PIF from basin Standay 1 Standay 1 1 1 1 1 1 1 1 1 1 1 1 1	innel fine screen ngd lly cleaned scre ed hes ngd ngd set by 54 feet t with 2 feet fi 000 gallons concrete ing rsepower w weir to pump cn)	ns, perforated plate
Required Effluent Quality   B0D5     Anticipated Filter Effluent Quality   B0D5     Headworks (2002 Construction)   Type of screens   In-cha     Number of screens   1     Peak flow capacity, each   9.25 m     Bypass screen   Manua     Screenings washing   Yes     Screenings compaction   Yes     Screenings compaction   Yes     Screenings compaction   Yes     Number of flumes   1     Throat width   24 inn     Peak flow capacity   21.4 m     Minimum flow capability   0.27 m     Aeration Basin   (1980 Construction)     Dimensions   Size (bottom of basin)   200 f     Side slopes (noriz:vert)   2:1     Maximum side water depth   10 feet     Basin volume, maximum   1,300,     Basin liner   Asphal     Aerators   Type     Type   Aspira     Number   6     Horsepower, each   10 ho     Basin outlet   Overfic     Transfer Pump Station (2002 Constructi     Estimated PIF from basin	innel fine screen ngd lly cleaned scre ed hes ngd ngd set by 54 feet t with 2 feet fi 000 gallons concrete ing rsepower w weir to pump cn)	ns, perforated plate
Type of screens in-chi Number of screens 1 Peak flow capacity, each 9.25 if Bypass screen Manua Screenings washing Yes Screenings compaction Yes Screenings compaction Yes Screenings compaction 2 Headworks, influent flow measurement Number of flumes 1 Throat width 24 into Peak flow capacity 21.4 if Minimum flow capability 0.27 if Aeration Basin (1980 Construction) Dimensions Size (bottom of basin) 200 ff Side slopes (horizvert) 2:1 Maximum side water depth 10 fet Basin volume, maximum 1,300, Basin liner Asphal Aerators Type Aspira Number 6 Horsepower, each 0 ho Basin outlet Overflot Transfer Pump Station (2002 Construction) Estimated PIF from basin 9.25 r Main pump type Centrif Main pump type Centrif	ngd lly cleaned scre ed hes ngd ngd set by 54 feet t with 2 feet fr 000 gollons concrete ing rsepower w weir to pump on)	reeboord
Number of screens   1     Peak flow capacity, each   9.25 m     Bypass screen   Manual     Screenings washing   Yes     Screenings compaction   Yes     Septage   Exclud     Headworks, influent flow measurement   1     Number of flumes   1     Throat width   24 inc     Peak flow capacity   21.4 m     Minimum flow capability   0.27 m     Aeration Basin   (1980 Construction)     Dimensions   Side slopes (horizivert)   211     Maximum side water depth   10 feat     Basin volume, maximum   1,300,     Basin liner   Aspirat     Number   6     Horsepower, each   10 ho     Dimension suitet   Overflat     Transfer Pump Station (2002 Constructi   Estimated PIF from basin   9.25 m     Main pump type   Centrif   Main pumps   Operating   1     Main pu	ngd lly cleaned scre ed hes ngd ngd set by 54 feet t with 2 feet fr 000 gollons concrete ing rsepower w weir to pump on)	reeboord
Number of flumes   1     Throat width   24 inc     Peak flow capacity   21.4 inc     Minimum flow capability   0.27 inc     Aeration Basin   (1980 Construction)     Dimensions   Size (bottom of basin)   200 fl     Side slopes (horiz:vert)   2:1     Maximum side water depth   10 feet     Basin volume, maximum   1,300,     Basin liner   Asphal     Aerators   Type     Number   6     Horsepower, each   10 ho     Basin outlet   Overflot     Transfer Pump Station (2002 Constructi)   9.25 r     Main pump type   Centrif     Main pump type   Centrif     Main pump type   1     Operating   1     Standoy   1	ngd ngd t with 2 feet fi 000 gallons concrete ing rsepower w weir to pump cn)	
Aeration Basin (1980 Construction)     Dimensions     Size (bottom of basin)   200 f     Side slopes (noriz:vert)   2:1     Maximum side water depth   10 fee     Basin volume, maximum   1,300,     Basin liner   Asphal     Aerators   Type     Number   6     Horsepower, each   10 ho     Basin outlet   Overflo     Transfer Pump Station (2002 Constructi)   9.25 r     Main pump type   Centrif     Main pump type   Centrif     Main pump type   1     Operating   1     Standby   1	eet by 54 feet t with 2 feet fi 200 gallons concrete ing rsepower w weir to pump on)	
Dimensions Size (bottom of basin) 200 f. Side slopes (horiz:vert) 2:1 Maximum side water depth 10 fec Basin volume, maximum 1,300, Basin liner Asphal Aerators Type Aspira Number 6 Horsepower, each 10 ho Basin outlet Overfice Transfer Pump Station (2002 Constructi Estimated PIF from basin 9.25 r Main pump type Centrif Main pump type Centrif Main pumps 0 Operating 1 Standay 1	t with 2 feet fi 000 gallons concrete ing rsepower w weir to pump on)	
Transfer Pump Station (2002 Constructi Estimated PIF from basin 9.25 r Main pump type Centrif Main pumps Operating 1 Standby 1	on)	
Estimated PIF from basin 9.25 r Main pump type Centrif Main pumps 0 Operating 1 Standby 1		
Jockey pump type Centril Jockey pump number 1 Jockey pump capacity (one forcemain) 2500 2100 Station peck capacity 7800 Required operating volume 4060 Wet well levels 286.0	ugal submersible gpm at 51 ft t ugal submersible gpm at 49 ft t gpm at 56 ft t gpm (11.23 mg gal ft	dh e w/vfd dh and dh
Minimum W.S. El 272.0 Operating volume 55,820	gal (includes	aeration basin)
Transfer Forcemain (2002 Construction)		
MaterialHDPESize18-inLength1630Velocity at 2500 gpm3.2 feVelocity at 2100 gpm2.6 feVelocity at 7800 gpm4.9 feOutlet to lagoon 11	et/sec et/sec	

Lagoon No. 1 (1980 Construction) Lagoon dimensions 11.4 acres (at 6-foot (average) depth) 12 feet with 3 feet of freeboard Surface area Maximum depth Working depth (max. To min.) 9 feet 137 acre-feet (45 mg) Lagoon volume, maximum Lagoon liner Native clay Aeration None Outlet Size 10-inch Capacity Surface weir and fixed pipe on bottom of lagoon Туре Lagoon No. 2 (1980 Construction) Lagoon dimensions Media 13.6 acres (at 6-foot average depth) 12 feet with 3 feet of freeboard Surface area Maximum depth Working depth 9 feet 163 acre-feet (53 mg) Lagoon volume, maximum Lagoon liner Native clay Outlet 14-inch Size Fixed pipes at two depths Туре Dissolved Air Flotation (DAF) (1980 Construction) 2.0 mgd Capacity Tank Diameter 31 feet Surface area 750 square feet (sf) Media Hydraulic loading rate 2.59 gpm/sf, including recycle Hydraulic capacity Chemical feed rates 2.80 mgd, including recycle 75 - 150 mg/l (not used) Alum Soda 37 - 75 mg/l (not used) 35 – 70 mg/l Polyaluminum Chloride Acid 0 - 10 mg/l (not used)Operating parameters Pressurized recycle flow 350 to 700 gpm 45 to 80 pounds per square inch (psi) Operating pressure Maximum horizontal velocity 3.1 feet per second 2290 pounds dry solids, 15,300 gallons Maximum daily sludge Recycle Pumps Number 2 20 HP Size Flow 350 gpm Recycle Flow Meter (Existing) Type Propeller 6 Inch Size 0-2 MGD Ranae Influent Flow Meter (FM-2) (2007 Construction) Electromagnetic Insertion Type Туре 12 Inch Size 0-10 MGD Range Dissolved Air Flotation (DAF) (2007 Construction) Capacity 2.0 mgd Tank Diameter 38 feet Surface area 1,075 square feet (sf) 2.0 gpm/sf, including recycle Maximum surface loading rate Hydraulic capacity 3.1 mgd, including recycle Chemical feed rates 35 – 70 mg/l Polyaluminum Chloride Operating parameters 350-700 gpm Pressurized recycle flow Operating pressure Maximum horizontal velocity 125 PSI <3.1 FPS 1,670 pounds dry solids, 10,000 gallons Maximum daily sludge Recycle Pumps Number 2 25 HP Size Flow 350 gpm Recycle Flow Meter (FM-3) Propeller Туре Size 6 Inch Range 0-2.5 MGD Influent Flow Meter (FM-1) Doppler Туре Size 14 Inch

0-14 MGD

 Image: Signed bit in the second se

Range

This drawing is full size when 22"x 34" or is reduced to holf size when 11"x17".

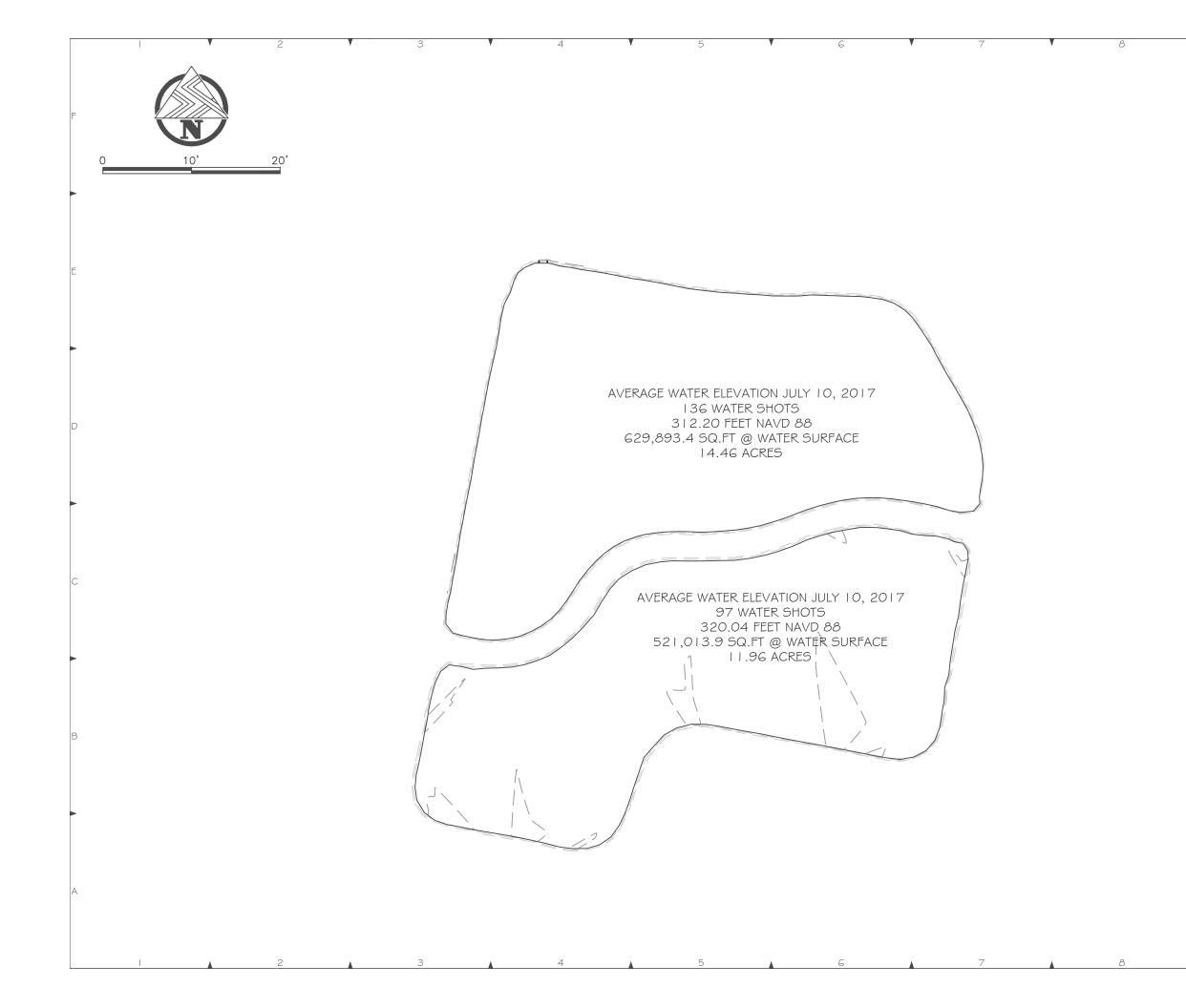
Plant Air (Proposed) Air Compressor Rotary Screw 15 HP Type Size Output 51 ACFM Operating Pressure 125 psig Gravity Filters (1980 Construction) - To Be Abandoned 2.1 mgd Number of filters 310 square feet Surface area, total 5 gpm/sf 2.2 mgd Maximum loading rate Hydraulic capacity Туре Gravel, sand, and anthracite cool Depth 22" gravel, 9" sand, 21" coal Backwash / surface wash Automatic on timer or pressure differential Backwash rate 20 gpm/sf 103 gpm Surface wash Gravity Filters (2007 Construction) Capacity Number of filters 4.0 mgd Surface area, total 573 square feet Maximum loading rate 4.85 gpm/sf Hydraulic capacity 4.0 mgd Gravel, sand, and anthracite coal Туре 12" silica sand, 24" anthracite coal Depth Manual, timed or pressure differential Backwash control 15 gpm/sf 2,147 gpm 4 - 8 minutes Backwash rate Backwash flow (1 filter) Backwash duration Backwash volume 8,600 - 17,200 gallons Air Scour Blower Rotary Positive Displacement Туре Size 15 HP 3.0 scfm/sf 429 scfm @ 4 psig Air scour rate Air Scour flow (1 filter) Backwash Flow Meter (FM-5) **Transit Time** Туре Size 16 Inch 0-17 MGD Range Filter Effluent Flow Meter (FM-4) Туре **Transit Time** Size 18 Inch Range 0-23 MGD

#### 2008 NPDES PERMIT RENEWAL Figure 3 DESIGN DATA 1

Disinfection (1980 Constructi Type	150-pound gas cylinders	Material	00 & 2006 Constructio PVC and HDPE
Chlorinator capacity Feed rate, minimum	100 ppd 30 to 50 pounds / million gallons	Size (Nominal Inside diameter)	24 inches
Residual, minimum	2 mg/l	Length Capacity	27,000 feet (approx. entire len
Chlorine Contact Chamber Sidewater depth	4 feet with 1 foot of freeboard	Design Capacity Design Velocity	10.1 mgd 5.0 fps
Volume Length to width ratio	67,500 gallons 24	Approx. Operating Range of Flows Operating Velocities	1.0 to 4.0 mgd 0.5 to 2.0 fps
-	Construction) — To Be Abandoned		
Size and material	18-inch concrete	Discharge Monitoring Structure (2	
Length Single port	270 feet	Dechlorination (for surface water discharge Feed Solution	conditions only) Ascorbic Acid
Туре	Perpendicular to streamflow	Chemical Feed Pumps	2 – 13 gph (with 1000:1 Turn
Stream gauge Type	Box culvert with 10' weir	Feed Control Effluent Sampler:	Flow and CI2 residual paced
Measurement	Manual staff guage	Type: Flow Paced or Time Composite Continuous Monitoring/Recording:	For Bacteria BOD, TSS, NH3, p
		Temperature	Probe.
Effluent/Irrigation Pump Stat	tion (2000 Construction)	DO Chlorine	Probe Residual Analyzer (2)
Pumps		Flow Measurement: Type	Electromagnetic Multi-port Inse
Type No. Of pumps	Vertical turbine 2 + 1 future	Size	12-inch
Capacity	500-7,000 gpm	Molalla River Outfall (2006 Const	,
Motor horsepower Motor control	300 bhp VFD	Material (for outfall extension and diffuser) Size (Nominal ID)	HDPE 24 inches
Wet well Type	12-ft id manhole	Length	23 feet
Operating volume, pump 1 on to pump off	3,800 gal	Diffuser Design Number of Ports	Three (Duckbill)
Level control	Pressure transducer	Diameter of Port Minimum Summer Submergence	Eight inches One Inch
Lift station (In Plant) Pump type	Submersible	Minimum Winter Submergence	12 Inches
No. Of pumps	2		
Capacity, each pump Motor horsepower	175 gpm at 15 ft tdh 3 bhp		
	1750 100		
Speed Wet well	1750 rpm 6-ft ID manhole		
Speed Wet well Level control	6-ft ID manhole Pressure transducer		
Wet well	6-ft ID manhole Pressure transducer		
Wet well Level control Plant Standby Power Generat Reliability Class Location Type Size	6-ft ID manhole Pressure transducer tor (2000 Construction) I Eff/Irr Pump Station Diesel Engine 750 KW		
Wet well Level control Plant Standby Power Generat Reliability Class Location Type	6-ft ID manhole Pressure transducer tor (2000 Construction) I Eff/Irr Pump Station Diesel Engine		
Wet well Level control Plant Standby Power General Reliability Class Location Type Size Transfer Switch	6-ft ID manhole Pressure transducer tor (2000 Construction) I Eff/Irr Pump Station Diesel Engine 750 KW		
Wet well Level control Plant Standby Power General Reliability Class Location Type Size Transfer Switch Plant Alarm System	6-ft ID manhole Pressure transducer tor (2000 Construction) I Eff/Irr Pump Station Diesel Engine 750 KW		
Wet well Level control Plant Standby Power General Reliability Class Location Type Size Transfer Switch	6-ft ID manhole Pressure transducer tor (2000 Construction) I Eff/Irr Pump Station Diesel Engine 750 KW Automatic		
Wet well Level control Plant Standby Power General Reliability Class Location Type Size Transfer Switch Plant Alarm System	6-ft ID manhole Pressure transducer tor (2000 Construction) I Eff/Irr Pump Station Diesel Engine 750 KW Automatic		
Wet well Level control Plant Standby Power General Reliability Class Location Type Size Transfer Switch Plant Alarm System	6-ft ID manhole Pressure transducer tor (2000 Construction) I Eff/Irr Pump Station Diesel Engine 750 KW Automatic		
Wet well Level control Plant Standby Power General Reliability Class Location Type Size Transfer Switch Plant Alarm System	6-ft ID manhole Pressure transducer tor (2000 Construction) I Eff/Irr Pump Station Diesel Engine 750 KW Automatic		
Wet well Level control Plant Standby Power General Reliability Class Location Type Size Transfer Switch Plant Alarm System	6-ft ID manhole Pressure transducer tor (2000 Construction) I Eff/Irr Pump Station Diesel Engine 750 KW Automatic		
Wet well Level control Plant Standby Power General Reliability Class Location Type Size Transfer Switch Plant Alarm System	6-ft ID manhole Pressure transducer tor (2000 Construction) I Eff/Irr Pump Station Diesel Engine 750 KW Automatic		
Wet well Level control Plant Standby Power General Reliability Class Location Type Size Transfer Switch Plant Alarm System	6-ft ID manhole Pressure transducer tor (2000 Construction) I Eff/Irr Pump Station Diesel Engine 750 KW Automatic		
Wet well Level control Plant Standby Power General Reliability Class Location Type Size Transfer Switch Plant Alarm System	6-ft ID manhole Pressure transducer tor (2000 Construction) I Eff/Irr Pump Station Diesel Engine 750 KW Automatic		
Wet well Level control Plant Standby Power General Reliability Class Location Type Size Transfer Switch Plant Alarm System	6-ft ID manhole Pressure transducer tor (2000 Construction) I Eff/Irr Pump Station Diesel Engine 750 KW Automatic		
Wet well Level control Plant Standby Power General Reliability Class Location Type Size Transfer Switch Plant Alarm System	6-ft ID manhole Pressure transducer tor (2000 Construction) I Eff/Irr Pump Station Diesel Engine 750 KW Automatic		
Wet well Level control Plant Standby Power General Reliability Class Location Type Size Transfer Switch Plant Alarm System	6-ft ID manhole Pressure transducer tor (2000 Construction) I Eff/Irr Pump Station Diesel Engine 750 KW Automatic		
Wet well Level control Plant Standby Power General Reliability Class Location Type Size Transfer Switch Plant Alarm System	6-ft ID manhole Pressure transducer tor (2000 Construction) I Eff/Irr Pump Station Diesel Engine 750 KW Automatic		
Wet well Level control Plant Standby Power General Reliability Class Location Type Size Transfer Switch Plant Alarm System	6-ft ID manhole Pressure transducer tor (2000 Construction) I Eff/Irr Pump Station Diesel Engine 750 KW Automatic		
Wet well Level control Plant Standby Power General Reliability Class Location Type Size Transfer Switch Plant Alarm System	6-ft ID manhole Pressure transducer tor (2000 Construction) I Eff/Irr Pump Station Diesel Engine 750 KW Automatic		

					DATE: JAN 2007	CIVIL REVIEW	
					DESIGNED; ECN	ARCH	
					DRAWN: BON		
					CHECKED: JOH	STRUC REVIEW	
REV DATE	DES	CHECK	APPROVALS	REVISION DESCRIPTION	APPROVED: JAM	MECH REVIEW	503-684-9097 Fax: 503-598-0583

### 2008 NPDES PERMIT RENEWAL Figure 4 DESIGN DATA 2



	PREPARED BY:     R:\PDG Standards\Logo's\pdg_logo_color.jpg     DATE SIGNED:     PREPARED FOR:     #####     REVISIONS     NO.   DESCRIPTION     DATE SIGNED:     PREPARED FOR:     #####     HORIZ DATUM:     ####     HORIZ DATUM:     ####     DRAVE:     APPROVED:					
-	R: \PDG Standards\Logo's\pdg_logo_color.jpg					
	DATE SIGNED.					
	REVISIONS NO. DESCRIPTION DATE BY					
	PPO IECT NO. ####					
	HORIZ DATUM: ####					
	VERT SCALE: AS SHOWN					
	DRAWN: ####					
	SHEET TITLE					
	####					

### Appendix C

Data Collection Table

#### City of Molalla Lagoon Leak Test Data Sheet

D-1-IT	Pump Station Meter	DAF No. 1	Meter (MG)	DAF No. 2	Meter (MG)		Temp	Biosolids Haul	Staff Gaug	ge Reading	]
Date/Time	(Totalizer - Gal.)	8:00 am Total	12:00 am Total	8:00 am Total	12:00 am Total	Precipitation (In.)	(°F)	Away Total (MG)	Lagoon 1	Lagoon 2	]
7/10 0800	358848216	0	$\Theta$	0	Ð	-0-	70	0	58.55"/9.4	10.1	
1/11 0800	359082681	0.095	0.209	0.197	0.450	-0-	67	0	58.55"	-2.25"	
1/12 0800	359970920	0.095	0.259	0.243	0.654	Ð	67	Ð	59.75"	-2.50	
7/13 0800	360869830	0.099	0.261	0.276	0.656	0	63	0	59.00 "	. 4.25	
7/14 0800	361722693	0.097	0,283	0.212	0.739	Ð	66	Ð	58 15/16"	- 12/10"	
7/15 0800	362613602	0.081	0.282	0.178	0.632	Ð	67	Ð	59 15/16"	- 18/16"	
7/16 0810	363429654	Ð	$\Theta$	0.007	Ð	0	66	Ð	59 15/16"	+5 12/16"	
7/17 0800	364345842	0	Ð	0	Ð	Ð	67	0	60.00''	+ 5,25"	
7/18 0800	365211963	0.081	0.228	0.306	0.705	Ð	68	0	59.50"	-5 3/16"	SHUT
7/19 0800	367021890	0.075	0.263	0,167	0.570	0	66	Ð	57.00"	- 1.00"	Be
7/20 0800	369976693	0.076	0.222	0,400	0.709	Ð	66	Ð	53.25"	- 4 8/16"	BP.
-										· ·	

### Appendix D

Data Entry and Analysis Spreadsheet

### City of Molalla 2017 Lagoon Leak Test July-17 <u>Transfer Pump Station Meter Readings</u>

	Totalized Meter Readings (gal)	Daily Flow		
	8:00:00 AM	(gal)	(in)	Test Day
Monday, July 10, 2017	358,848,216			
Tuesday, July 11, 2017	359,082,681	234,465	0.33	Day 1
Wednesday, July 12, 2017	359,970,920	888,239	1.24	Day 2
Thursday, July 13, 2017	360,869,830	898,910	1.25	Day 3
Friday, July 14, 2017	361,722,693	852,863	1.19	Day 4
Saturday, July 15, 2017	362,613,602	890,909	1.24	Day 5
Sunday, July 16, 2017	363,429,654	816,052	1.14	Day 6
Monday, July 17, 2017	364,345,842	916,188	1.28	Day 7
Tuesday, July 18, 2017	365,211,963	866,121	1.21	Day 8
Wednesday, July 19, 2017	367,021,890	1,809,927	2.52	Day 9
Thursday, July 20, 2017	369,120,278	2,098,388	2.92	Day 10
Friday, July 21, 2017				
Totals		10,272,062	14.32	

Lagoon Storage (gal/in)

717,399

City of Molalla 2017 Lagoon Leak Test July-17 Lagoon Levels

Lagoon Properties at Test Depth								
	Depth	epth Volume Slope of Staff Gau						
	(ft)	Area	(gal/ft)	(gal/in)	(deg)	(rad)		
Lagoon #1	9.4	521,014	3,897,185	324,765	N/A	N/A		
Lagoon #2	10.1	629,893	4,711,603	392,634	16	0.27925268		
Total		1,150,907	8,608,787	717,399	N/A	N/A		

Lagoon No. 1								
	Reading	Change	in Depth	Depth Change ir		n Volume		
	(in)	(in)	(ft)	(ft)	(g	al)		
Monday, July 10, 2017	58.55			9.40				
Tuesday, July 11, 2017	58.55	0.000	0.000	9.40	0	Day 1		
Wednesday, July 12, 2017	59.75	-1.200	-0.100	9.30	-389,718	Day 2		
Thursday, July 13, 2017	59.00	0.750	0.063	9.36	243,574	Day 3		
Friday, July 14, 2017	58.94	0.060	0.005	9.37	19,486	Day 4		
Saturday, July 15, 2017	59.31	-0.370	-0.031	9.34	-120,163	Day 5		
Sunday, July 16, 2017	59.94	-0.630	-0.053	9.28	-204,602	Day 6		
Monday, July 17, 2017	60.00	-0.060	-0.005	9.28	-19,486	Day 7		
Tuesday, July 18, 2017	59.50	0.500	0.042	9.32	162,383	Day 8		
Wednesday, July 19, 2017	57.00	2.500	0.208	9.53	811,913	Day 9		
Thursday, July 20, 2017	53.25	3.750	0.313	9.84	1,217,870	Day 10		
Totals		5.30	0.442		1,721,257			

Lagoon No. 2								
	Reading	Change in Depth		Depth	Change ir	nange in Volume		
	(in)	(in)	(ft)	(ft)	(ga	al)		
Monday, July 10, 2017	0.00			10.10				
Tuesday, July 11, 2017	-2.25	-0.62	-0.12	9.98	-243,432.80	Day 1		
Wednesday, July 12, 2017	-2.50	-0.69	-0.08	9.90	-270,917.15	Day 2		
Thursday, July 13, 2017	-4.25	-1.17	-0.09	9.81	-459,381.26	Day 3		
Friday, July 14, 2017	-1.13	-0.31	-0.13	9.68	-121,716.40	Day 4		
Saturday, July 15, 2017	-1.50	-0.41	-0.03	9.65	-160,979.76	Day 5		
Sunday, July 16, 2017	5.50	1.52	0.13	9.78	596,803.00	Day 6		
Monday, July 17, 2017	5.25	1.45	0.12	9.90	569,318.65	Day 7		
Tuesday, July 18, 2017	-5.19	-1.43	-0.12	9.78	-561,465.98	Day 8		
Wednesday, July 19, 2017	-1.00	-0.28	-0.02	9.76	-109,937.39	Day 9		
Thursday, July 20, 2017	-4.50	-1.24	-0.10	9.66	-486,865.61	Day 10		
Totals		-3.18	-0.265		-1,248,575			

#### City of Molalla 2017 Lagoon Leak Test July-17 DAF Meter Readings

		DAF No. 1				DAF No. 2				
	Meter Rea	dings (MG)	Daily Flow		Meter Readings (MG)		Daily Flow		N	
	8:00:00 AM	11:55:00 PM	(MG)	(in)	Test Day	8:00:00 AM	11:55:00 PM	(MG)	(in)	Test Day
Monday, July 10, 2017	0	0				0	0			
Tuesday, July 11, 2017	0.095	0.209	0.095	0.13	Day 1	0.197	0.45	0.197	0.27	Day 1
Wednesday, July 12, 2017	0.095	0.259	0.209	0.29	Day 2	0.243	0.654	0.496	0.69	Day 2
Thursday, July 13, 2017	0.099	0.261	0.263	0.37	Day 3	0.276	0.656	0.687	0.96	Day 3
Friday, July 14, 2017	0.097	0.283	0.259	0.36	Day 4	0.212	0.739	0.592	0.83	Day 4
Saturday, July 15, 2017	0.081	0.282	0.267	0.37	Day 5	0.178	0.632	0.705	0.98	Day 5
Sunday, July 16, 2017	0.000	0.000	0.201	0.28	Day 6	0	0.000	0.454	0.63	Day 6
Monday, July 17, 2017	0.000	0.000	0.000	0.00	Day 7	0.000	0.000	0.000	0.00	Day 7
Tuesday, July 18, 2017	0.081	0.228	0.081	0.11	Day 8	0.306	0.705	0.306	0.43	Day 8
Wednesday, July 19, 2017	0.075	0.263	0.222	0.31	Day 9	0.167	0.570	0.566	0.79	Day 9
Thursday, July 20, 2017	0.076	0.222	0.264	0.37	Day 10	0.406	0.709	0.809	1.13	Day 10
Totals			1.861	2.59				4.812	6.71	

Lagoon Storage (gal/in) 717,399

### City of Molalla 2017 Lagoon Leak Test July-17 <u>Evaporation</u>

	Evaporation Pan Depth			
	Logger Reading	Change in Level	<b>T</b> ( <b>D</b>	
	8:00 AM	J	Test Day	
	(in)	(in)		
Monday, July 10, 2017	8.008			
Tuesday, July 11, 2017	7.704	-0.304	Day 1	
Wednesday, July 12, 2017	7.464	-0.240	Day 2	
Wed Water Level Adjustment	7.238			
Thursday, July 13, 2017	7.033	-0.205	Day 3	
Friday, July 14, 2017	6.863	-0.170	Day 4	
Saturday, July 15, 2017	6.630	-0.233	Day 5	
Sunday, July 16, 2017	6.358	-0.272	Day 6	
Monday, July 17, 2017	6.111	-0.247	Day 7	
Tuesday, July 18, 2017	5.860	-0.251	Day 8	
Wednesday, July 19, 2017	5.590	-0.270	Day 9	
Thursday, July 20, 2017	5.330	-0.260	Day 10	
Total		-2.452		

Precipitation & Pan Coefficient Adjustments				
Evaporation (in)	-2.452			
Pan Coefficient	0.8			
Corrected Evaporation (in)	-1.962			
Lagoon Storage (gal/in)	717,399			
Evaporation Total (gal)	-1,407,250			

### City of Molalla 2017 Lagoon Leak Test July-17 <u>Volume Summary</u>

	Total Flow	Avg. Daily Flow
Flow Element	10 Day Test	10 Day Test
	Period	Period
	(gal.)	(gal./day)
Lagoon Influent Flow	10,272,062	1,027,206
Precipitation	0	0
Lagoon 1 Storage	-1,721,257	-172,126
Lagoon 2 Storage	1,248,575	124,857
Lagoon Effluent (DAF No. 1)	-1,861,000	-186,100
Lagoon Effluent (DAF No. 2)	-4,812,000	-481,200
Evaporation	-1,407,250	-140,725
Balance/Seepage	1,719,130	171,913

### Appendix E

Oregon DEQ Guidelines for Estimating Leakage from Existing Sewer Lagoons

### **State of Oregon**

### **Department of Environmental Quality Guidelines**

**Guidelines for Estimating Leakage from Existing Sewage Lagoons** 

PURPOSE AND SCOPE EXCLUSIONS GENERAL APPROACH EQUIPMENT REQUIREMENTS MEASUREMENTS AND CALCULATIONS REPORT FORMAT ANNUAL WATER BALANCE

NEW LAGOONS

### PURPOSE AND SCOPE

These guidelines provide for relatively inexpensive test equipment and procedures to be used for prioritizing problem lagoons used for treating domestic sewage. Such tests are not definitive. They should be considered preliminary and approximate.

Tests based on these guidelines can only indicate whether the seal on an existing lagoon probably remains intact, or approximately how much it may be leaking. Preliminary tests of this type are not suitable for sewage lagoons where there is a strong likelihood of contamination, or an immediate urgency to protect a priority aquifer.

### EXCLUSIONS

Such preliminary testing is not suitable for various types of lagoons which may contain stronger wastes than sewage. For example, leak tests for sludge, septage, strong industrial wastes, and landfill leachate lagoons may warrant a higher level of accuracy. To attain greater accuracy entails considerable time and expense, requires more equipment to develop wind and temperature records, and involves calculations outside the scope of these guidelines.

Such accuracy is seldom warranted for sewage lagoons. In critical groundwater pollution situations, where lagoon seepage is a known concern, immediate installation of monitoring wells and a formal program of groundwater monitoring are normally warranted. In such situations, no program of leak testing is probably accurate enough to substitute for direct groundwater monitoring. Leak testing would only delay the definitive determinations that must be made.

### **GENERAL APPROACH**

The general objective of a leak test is to estimate the average rate of seepage through the bottom of the lagoon. Normally each lagoon cell is isolated and tested separately, which better pinpoints the location of any major leaks. The rate of seepage is expressed in inches per day or centimeters per second.

Leak testing should be restricted to July and August, when rainfall is minimal and the ground is dry enough to exclude significant runoff. Tests conducted at other times will have more variables and may underreport seepage due to runoff effects.

To obtain reasonable precision, each cell of a lagoon should be isolated and tested over a period of 10 - 15 days. Cell depth and pan evaporation measurements should be taken daily. If the lagoon cell cannot be isolated, then daily influent/effluent flows must also be measured. Daily measurements are preferred over weekly to improve precision and to minimize random measurement errors.

Lagoon liquid depth should suit the purpose of the test. To determine average seepage rates, lagoons should be at average operating depth.

In priority areas, any rate of seepage greater than zero may warrant direct sampling and monitoring of the groundwater. Seepage of 1/8" per day or less is normal. However, this low rate can cause groundwater contamination where lagoon contents are strong and background levels are high quality. Seepage exceeding 1/4" per day indicates a seal failure, or absence of adequate initial seal.

### **EQUIPMENT REQUIREMENTS**

Each cell of a lagoon needs to be equipped with a staff gauge for level measurements. Stilling wells to dampen wave action are recommended, and will allow a staff gauge to be read to 1/8" - 1/16". Precipitation can be measured to about 1/100" with a good rain gauge. Evaporation can be measured to roughly 1/1000" with a hook gauge.

The following specifications for rainfall and evaporation equipment are based on Weathertronics equipment manufactured by Qualimetrics, Inc. of Sacramento, and available in Oregon through International Reforestation Supply, Eugene (345-0597). Equivalent equipment is acceptable.

1. <u>Rain Gauge</u>. Qualimetrics Model 6330. This is a plastic gauge with 11" capacity and 0.01" graduations, designed for post mounting.

2. <u>Evaporation Pan.</u> Qualimetrics Model 6821. This is a standard US Weather Bureau steel pan, 47.5" diameter by 10" deep.

3. <u>Hook Gauge.</u> Qualimetrics Model 6831. This is a brass gauge with 0.02" graduations.

To obtain accurate measurements, the equipment needs to be set up level and plumb in an unsheltered area near the lagoon. Equipment may have to be fenced to exclude animals.

The above list is a minimum. Various equipment needed to attain higher levels of accuracy is not listed. For example:

- · Recording anemometer
- $\cdot$  Max/min thermometers for air, for the evaporation pan, and for the lagoon surface

- · Upwind and downwind evaporation pans
- · Barometric pressure

If such equipment is available, its use will add precision and accuracy to the results. However, its use is not mandatory for preliminary leak tests used to screen and prioritize existing sewage treatment lagoons.

### MEASUREMENTS AND CALCULATIONS

Measurements should be made on a schedule, at the same time each day, so that each set of data represents the duration of exactly one day. All measurements should be tabulated to aid calculation and reporting. We recommend using the attached form or a similar format.

Computations should be converted to compatible units of depth. Influent volume (gallons per day) is converted to inches per day through measurement of the actual water surface area. Rainfall will normally be near zero in July and August, but should be verified daily.

Evaporation will vary with wind and temperature. It should be measured daily, and the pan should be kept well filled.

Lagoon evaporation rates are invariably less than pan evaporation rates. Pan correction factors generally vary from 0.7 to 0.9. The larger the lagoon, the more its evaporation rate lags behind pan evaporation, so the smaller the numerical value of the pan correction factor.

In hot and windy summer weather, evaporation can be substantial. An erroneous pan correction factor can inject significant error. The result of computing seepage rates without any correction for pan evaporation is to overcalculate the evaporation rate. The effect of this error would be to underreport the seepage rate.

### **REPORT FORMAT**

Leakage reports should be short and to the point. The main conclusion is to estimate the seepage rate from each lagoon cell, and from the lagoon as a whole. The methodology and equipment need to be described briefly but thoroughly. A copy of all field measurements and calculations should be tabulated and attached as supporting documentation.

Reports should be certified and signed by a registered engineer or professional hydrologist.

### ANNUAL WATER BALANCE

The annual water balance prepared for each lagoon requires determinations of both seepage and evaporation. Leak tests performed according to these guidelines at average liquid depth can establish an average rate of seepage for the water balance. The rate of seepage will tend to vary with liquid level, and will remain constant if the level stays constant.

For the purpose of making water balance calculations, a monthly average evaporation rate should be obtained from local climatological records. Such records may then be applied with a suitable pan correction factor between 0.7 and 0.9, as previously described.

Rate of evaporation and pan correction factors both tend to vary throughout the year. To make accurate adjustments requires additional measurements be taken of all the pertinent factors. These include wind, water

temperature, air temperature, and atmospheric pressure. Pan evaporation corrections should conform to established calculation methods, as presented in standard hydrology texts.

### **NEW LAGOONS**

New sewage and sludge lagoons are designed to be effectively watertight and nearly leak-free. Lagoons which may jeopardize groundwater because of their contents, uses, or location are routinely installed with groundwater monitoring wells. In such applications, leak testing is not a practical or reliable alternative to direct monitoring of the groundwater.

All of the measurements in leak tests are approximations, especially liquid level, and the pan correction factor is usually a rough estimate. Consequently, seepage computed from a leak test cannot be used to prove or substantiate the existence of any actual leak. Leak testing as a basis for acceptance of lagoon construction is not feasible, too often has led to fruitless litigation, and should be discouraged.

As a practical matter, the engineer must design each lagoon for watertightness. Then the engineer must conduct thorough, intensive, and continuous construction inspection to verify that watertight construction is being attained. Inspection may include compaction, infiltrometer, smoke, and spark tests, and constant observation of workmanship and materials.

If leakage and contamination occurred from a properly inspected and certified lagoon, it would indicate a damaged liner or a failure of design. Assuming good design and inspection, the engineer's written certification of proper construction carries a presumption of watertightness. No leak testing program should be approved as a substitute for diligent construction inspection.

### **INQUIRIES**

Inquiries about these guidelines should be directed to DEQ regional water-quality plan review engineers.

DSM:LAGOON2.TST

Orig. V.93

Rev1. VIII.94

### LAGOON LEAK TEST

CITY OF \_\_\_\_\_

### CELL NO.\_\_\_\_ WATER SURFACE AREA\_\_\_\_\_

CELL WATER DEPTH @ TEST START\_\_\_\_\_ @ TEST END\_\_\_\_\_\_

Date	INFLUENT (in/day)	+PRECIP (in/day)	-EVAP (in/day)	- EFFLUENT (in/day)	=NET SEEPAGE

NOTES: